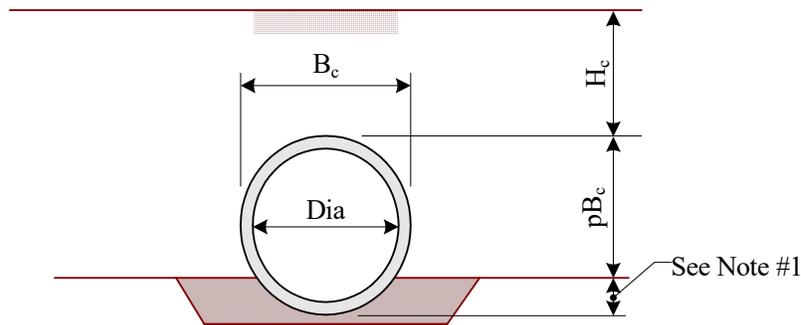


**Project:**  
**Subject:**  
**Design By:**

**Checked By:**

**References:**

1. EM 1110-2-2902, Conduits, Culverts and Pipes, 31 October 1997 (CH1 31 March 98)
2. Concrete Pipe Design Manual, American Concrete Pipe Association, April 2007
3. Design Data 1, Highway Live Loads on concrete Pipe, American Concrete Pipe Association, July 2009
4. ASTM C76, Reinforced Concrete Culvert, Storm Drain, And Sewer Pipe, 2010
5. AWWA Manual M9, Third Edition, 2011



**Given:**

Pipe Inside Diameter:  $\text{Dia} := 72 \cdot \text{in}$       Height of fill:  $H_c := 8.54 \cdot \text{ft}$

Pipe wall thickness based on ASTM C76 (Wall A, B or C):  $T_{\text{wall}} := 6 \cdot \text{in}$

Pipe outside diameter:  $B_c := \text{Dia} + 2 \cdot T_{\text{wall}}$        $B_c = 7.000 \text{ ft}$

Saturated Unit Weight of soil:  $\gamma := 133 \cdot \text{pcf}$

Hydraulic Factor:  $H_f := 1.3$       *The pipe is designed per Chapter 3 of Reference #1.*

Specific weight of water:  $\gamma_w := 62.4 \cdot \text{pcf}$

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### Determination of Bedding Factor:

#### Note:

1. Typical COE guide specification for bedding requirements for RCP are as follows:

*The bedding surface for the pipe shall provide a firm foundation of uniform density throughout the entire length of the pipe. When no bedding class is specified or detailed on the drawings, concrete pipe shall be bedded carefully in a **soil foundation accurately shaped and rounded to conform to the lowest one-fourth of the outside portion of circular pipe** for the entire length of the pipe. When necessary, the bedding shall be tamped. Bell holes and depressions for joints shall be only of such length, depth, and width as required for properly making the particular type of joint.*

2. The requirements above correspond to Ordinary Bedding (Fig.3-2b of Ref. #1) or Class C Bedding (Fig. B-16 of Ref. #2) which have a projection ratio  $\rho := 0.9$ .

From Table 3-2 of Ref#1

$$X_p := 0.840$$

$$X_a := .655$$

Table 3-2 of Ref #1

$$B_f := \frac{1.431}{X_p - \frac{X_a}{3}}$$

$$B_f = 2.302$$

EQ 3-1 of Ref # 1

### Determination of Earth Load:

See reference #1, para 5-4, para. 3-7a.(2)

$$W_E := 1.5 \cdot B_c \cdot (H_c \cdot \gamma)$$

$$W_E = 11926 \cdot \frac{\text{lbf}}{\text{LF}} \quad \text{Eq. 2-12 of Ref #1}$$

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**Determination of Live Load (HS 20-44) Pressure:**

*See reference #3, Table 3*

**Vehicle Traveling Perpendicular to the Pipe**

$$P := \begin{cases} 16000 \cdot \text{lb} & \text{if } H_c + 1.30B_c < 2.30 \cdot \text{ft} \\ 32000 \cdot \text{lb} & \text{if } 2.30 \cdot \text{ft} - 1.30B_c \leq H_c < 6.3 \cdot \text{ft} \\ 50000 \cdot \text{lb} & \text{otherwise} \end{cases}$$

$P = 50000 \text{ lb}$  <==== Critical Wheel Load

Surface Contact Area:  $a := 1.67 \text{ ft}$   $b := .83 \text{ ft}$

**Spread load Area at the top side of Pipe**

$$\text{Spread}_b := \begin{cases} (b + H_c) & \text{if } H_c + 1.30B_c < 2.30 \cdot \text{ft} \\ (b + H_c) & \text{if } 2.30 \cdot \text{ft} - 1.30B_c \leq H_c < 6.3 \cdot \text{ft} \\ (b + 4 \text{ ft} + H_c) & \text{otherwise} \end{cases}$$

$\text{Spread}_b = 13.370 \text{ ft}$

$$\text{Spread}_a := \begin{cases} (a + H_c) & \text{if } H_c + 1.30B_c < 2.30 \cdot \text{ft} \\ (a + 4 \text{ ft} + H_c) & \text{if } 2.30 \cdot \text{ft} - 1.30B_c \leq H_c < 6.3 \cdot \text{ft} \\ (a + 4 \text{ ft} + H_c) & \text{otherwise} \end{cases}$$

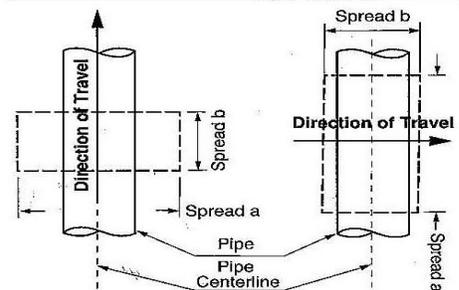
$\text{Spread}_a = 14.210 \text{ ft}$

**Determination of Live Load (HS 20-44) Pressure Continued:**

$A := \text{Spread}_a \cdot \text{Spread}_b$

$A = 189.988 \cdot \text{ft}^2$  <==== Critical Wheel Load Area

**Figure 6 Spread Load Area Dimensions vs Direction of Truck**



- L = dimension of A parallel to the longitudinal axis of pipe, feet  
 For vehicles traveling perpendicular to the pipe, L = spread a  
 For vehicles traveling parallel to the pipe, L = spread b
- S<sub>L</sub> = outside horizontal span of pipe, D<sub>o</sub>, or spread wheel load area, A, transverse to the longitudinal axis of pipe, whichever is less, feet

Project:  
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**Impact Factor:**

*See reference #3, Eqn 1*

$$IM := \begin{cases} 0 & \text{if } H_c \geq 8\text{ft} \\ \frac{33(1.0\text{ft} - .125H_c)}{100\text{ft}} & \text{otherwise} \end{cases} \quad IM = 0.000$$

**Average Pressure intensity**

*See reference #3, Eqn 3*

$$w := \frac{P \cdot (1 + IM)}{A} \quad w = 263.175 \cdot \text{psf}$$

**Total Live Load**

*See reference #3, Eqn 4 & 5*

Since the truck travels transverse to the pipe centerline:

$$L := \text{Spread}_a \quad L = 14.210 \text{ ft}$$

$$S_L := \begin{cases} \text{Spread}_b & \text{if } \text{Spread}_b \leq B_c \\ B_c & \text{otherwise} \end{cases} \quad S_L = 7.000 \text{ ft}$$

$$W_T := w \cdot L \cdot S_L \quad W_T = 2.618 \times 10^4 \text{ lbf}$$

$$L_e := L + 1.75 \left( \frac{3}{4} B_c \right) \quad L_e = 23.397 \text{ ft}$$

$$W_L := \frac{W_T}{L_e} \quad W_L = 1118.838 \cdot \frac{\text{lbf}}{\text{ft}}$$

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**Required D Load :**

*See reference #1, Pg. 3-6, Eqn 3-2*

$$D_{0.01} := \frac{H_f (W_L + W_E)}{B_f \text{Dia}}$$

$$D_{0.01} = 1227.870 \frac{\text{lbf}}{\text{LF} \cdot \text{ft}}$$



**USE**